

a. h. s. e.  
replace  
11-55

Figure 1

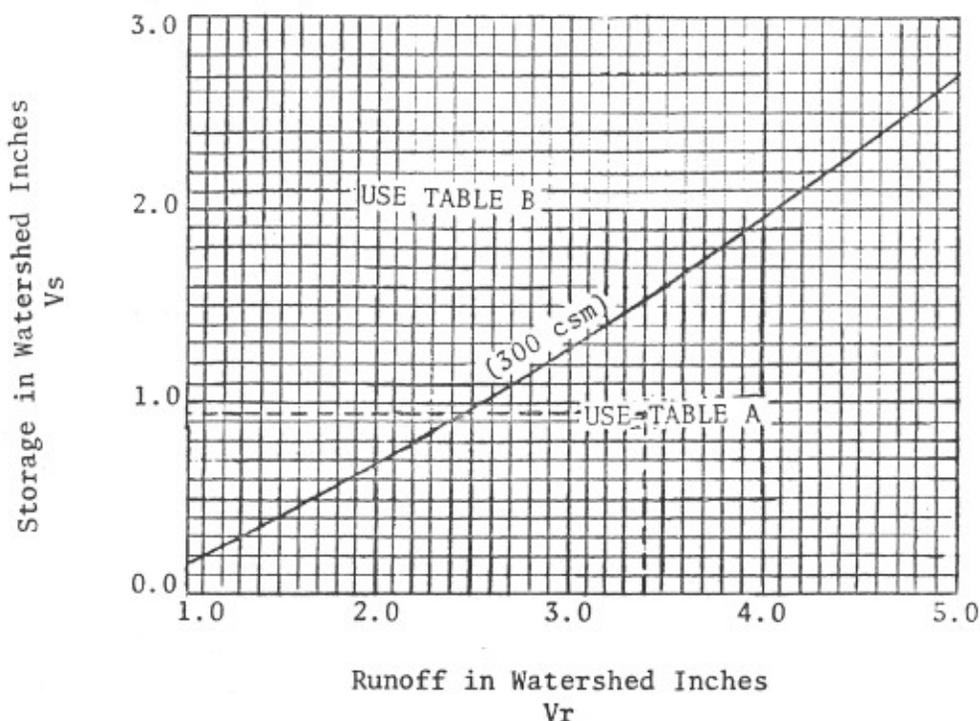


Figure 1 and Tables A and B can be used to determine the capacity of principal spillways considering temporary storage. Figure 1 is a plot of  $V_s$  versus  $V_r$  to determine which Table to use. Table A is for pipe flow structures with a discharge over  $0.47 \text{ ft}^3/\text{s/acre}$  (300 csm) and Table B is for pipe flow structures with a discharge under  $0.47 \text{ ft}^3/\text{s/acre}$ .

#### Description of Terms:

$V_s$  = Volume of temporary storage, acre-feet or in.

$V_r$  = Volume of runoff, acre-feet or in.

$Q_0$  = Required principal spillway discharge,  $\text{ft}^3/\text{s}$  (Table A) and  $\text{ft}^3/\text{s/acre}$  (Table B)

$Q_i$  = Peak flow from design storm,  $\text{ft}^3/\text{s}$

Exhibit 11-4 Estimate of principal spillway discharge allowing for temporary storage.

Table A

| $\frac{V_s}{V_r}$ |      |     |     |     |     |     |     |     |            |     |
|-------------------|------|-----|-----|-----|-----|-----|-----|-----|------------|-----|
|                   | 0.00 | .01 | .02 | .03 | .04 | .05 | .06 | .07 | .08        | .09 |
| 0.0               | 1.00 | .99 | .98 | .96 | .95 | .94 | .92 | .91 | .90        | .88 |
| 0.1               | .87  | .85 | .84 | .82 | .81 | .79 | .78 | .76 | .75        | .74 |
| 0.2               | .70  | .67 | .64 | .61 | .58 | .56 | .54 | .52 | <u>.50</u> | .48 |
| 0.3               | .47  | .45 | .44 | .42 | .41 | .40 | .39 | .38 | <u>.37</u> | .36 |
| 0.4               | .35  | .32 | .30 | .28 | .26 | .24 | .23 | .21 | .20        | .19 |
| 0.5               | .18  | .17 | .16 | .15 | .14 | .13 | .12 | .12 | .11        | .11 |
| 0.6               | .10  | .10 | .09 | .09 | .08 | .08 | .07 | .07 | .07        | .07 |
| 0.7               | .06  | .06 | .06 | .06 | .05 | .05 | .05 | .05 | .04        | .04 |
| 0.8               | .04  | .04 | .04 | .04 | .04 | .03 | .03 | .03 | .03        | .03 |

Table A: Values of  $Q_o/Q_i$  for pipe flow structures with a discharge over  $0.47 \text{ ft}^3/\text{s}/\text{acre}$  (300 csm).

Example #1:

Given:  $V_s = 5.9$  acre-feet or 0.94 in.  
 $V_r = 21.1$  acre-feet or 3.4 in.  
 $Q_i = 360 \text{ ft}^3/\text{s}$   
D.A. = 75 acres

Find:  $Q_o$

Solution: Find the point for  $V_s$  of 0.94 in. and  $V_r$  of 3.4 in. in figure 1. Since the point is below the line, use Table A.

$$\frac{V_s}{V_r} = \frac{0.94}{3.4} = 0.28 \text{ (} V_s \text{ and } V_r \text{ must be in same units)}$$

$$\frac{Q_o}{Q_i} = 0.50 \text{ (From Table A)}$$

$$Q_o = 0.50 \times Q_i = 0.50 \times 360 \text{ ft}^3/\text{s} = 180 \text{ ft}^3/\text{s}$$

Table B

VALUES OF  $Q_0$  IN  $\text{ft}^3/\text{s}/\text{acre}$ 

STORAGE IN WATERSHED INCHES

| $V_s$                      | $V_r$ | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | 1.0 | 1.2 | 1.4 | 1.6 | 1.8 | 2.0 | 2.2 | 2.4 | 2.6 | 2.8 | 3.0 | 3.2 | 3.4 |     |     |
|----------------------------|-------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| RUNOFF IN WATERSHED INCHES | 1.0   | .89 | .39 | .19 | .10 | .06 |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |
|                            | 1.2   |     | .62 | .55 | .20 | .12 | .08 | .05 | .04 |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |
|                            | 1.4   |     |     | .57 | .31 | .21 | .14 | .09 | .07 | .05 |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |
|                            | 1.6   |     |     |     | .57 | .33 | .23 | .16 | .11 | .08 | .06 |     |     |     |     |     |     |     |     |     |     |     |     |     |     |
|                            | 1.8   |     |     |     |     | .59 | .37 | .25 | .19 | .13 | .09 | .06 |     |     |     |     |     |     |     |     |     |     |     |     |     |
|                            | 2.0   |     |     |     |     |     | .64 | .42 | .27 | .21 | .14 | .09 | .06 |     |     |     |     |     |     |     |     |     |     |     |     |
|                            | 2.2   |     |     |     |     |     |     | .62 | .45 | .32 | .23 | .13 | .08 | .05 |     |     |     |     |     |     |     |     |     |     |     |
|                            | 2.4   |     |     |     |     |     |     |     | .65 | .49 | .34 | .20 | .12 | .08 | .05 |     |     |     |     |     |     |     |     |     |     |
|                            | 2.6   |     |     |     |     |     |     |     |     | .49 | .28 | .17 | .11 | .08 | .05 |     |     |     |     |     |     |     |     |     |     |
|                            | 2.8   |     |     |     |     |     |     |     |     | .69 | .41 | .25 | .16 | .11 | .08 | .05 |     |     |     |     |     |     |     |     |     |
|                            | 3.0   |     |     |     |     |     |     |     |     |     | .55 | .34 | .22 | .15 | .11 | .08 | .05 |     |     |     |     |     |     |     |     |
|                            | 3.2   |     |     |     |     |     |     |     |     |     |     | .49 | .31 | .20 | .14 | .10 | .08 | .05 |     |     |     |     |     |     |     |
|                            | 3.4   |     |     |     |     |     |     |     |     |     |     |     | .66 | .44 | .28 | .19 | .14 | .10 | .08 | .05 |     |     |     |     |     |
|                            | 3.6   |     |     |     |     |     |     |     |     |     |     |     |     | .56 | .39 | .26 | .19 | .14 | .10 | .08 | .05 |     |     |     |     |
|                            | 3.8   |     |     |     |     |     |     |     |     |     |     |     |     |     | .49 | .34 | .23 | .16 | .13 | .10 | .08 | .05 |     |     |     |
|                            | 4.0   |     |     |     |     |     |     |     |     |     |     |     |     |     |     | .65 | .45 | .31 | .22 | .17 | .13 | .09 | .07 | .05 |     |
|                            | 4.2   |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     | .55 | .41 | .28 | .20 | .16 | .12 | .09 | .07 |     |
|                            | 4.4   |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     | .49 | .36 | .25 | .21 | .14 | .12 | .09 |     |
|                            | 4.6   |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     | .62 | .45 | .33 | .27 | .19 | .15 | .12 |
|                            | 4.8   |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     | .55 | .43 | .33 | .23 | .19 | .14 |
|                            | 5.0   |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     |     | .49 | .39 | .31 | .23 | .18 |

DISCHARGE VALUES TO THE LEFT OF LINE ARE TO BE USED FOR INTERPOLATION ONLY.

HOWEVER, INTERPOLATED VALUES SHOULD NOT EXCEED  $.47 \text{ ft}^3/\text{s}/\text{acre}$ 

Table B is for pipe flow structures with a discharge under  $0.47 \text{ ft}^3/\text{s}/\text{acre}$  (300 csm).

Example #2:

Given:  $V_s = 34$  acre-feet or 1.6 in.  
 $V_r = 3.2$  in.  
 $Q_i = 420 \text{ ft}^3/\text{s}$   
 D.A. = 256 acres (drainage area)

Find:  $Q_0$

Solution: Use Table B (determined from Figure 1)  
 $Q_0 = 0.31 \text{ ft}^3/\text{s}/\text{acre}$  (from Table B)  
 $= 0.31 \text{ ft}^3/\text{s}/\text{acre} \times 256 \text{ acres} = 79 \text{ ft}^3/\text{s}$